

Montgomery County Board of Commissioners

Ordinance 2020 – 13

Amending Storm Water Management Policy

Whereas, Chapter 154 of the Montgomery County Code contains the Montgomery County Storm Water Management Policy; and

Whereas, James Peck, Montgomery County Engineer, has requested that certain amendments be made to the policy in order to better manage storm water drainage, clarify standards for construction of storm water facilities, and expedite reviews of proposed plans; and


Whereas, the Board of Commissioners has reviewed the proposed amendments and finds that the proposed should be adopted and that Chapter 154 of the County Code should be amended accordingly.

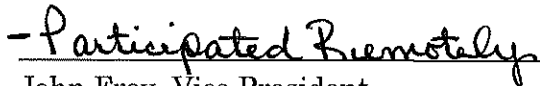
It is therefore ordained that Chapter 154 of the Montgomery County Code is amended to read as shown in Exhibit A.

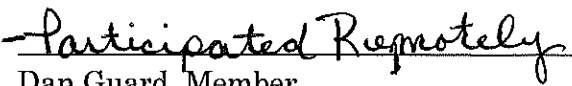
It is further ordained that all provisions of the County Code, which are not expressly modified by this ordinance will remain in full force and effect.

It is further ordained that this ordinance will become effective upon adoption. Adopted May 11, 2020.

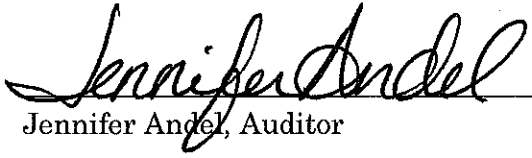
Montgomery County
Board of Commissioners:


James D. Fulwider, President


John Frey, Vice President


Dan Guard, Member

Attest:

A handwritten signature in cursive script that reads "Jennifer Andel". The signature is written in black ink and is positioned above a horizontal line.

Jennifer Andel, Auditor

Exhibit A

CHAPTER 154: STORMWATER DRAINAGE

Section

General Provisions

- 154.01 Authority and Jurisdiction
- 154.02 Purpose
- 154.05 Definitions
- 154.06 Disclaimer of liability
- 154.07 Violations
- 154.08 Exempt projects
- 154.09 Fees

Drainage Control Policy

- 154.20 Policy
- 154.21 Permits for construction
- 154.22 Information requirements
- 154.23 Determination of runoff quantities
- 154.24 Amount to be accommodated

Design Standards

- 154.35 General design
- 154.36 Manning equation
- 154.37 Minimum size
- 154.38 Grade
- 154.39 Alignment
- 154.40 Manholes
- 154.41 Inlets
- 154.42 Workmanship and materials
- 154.43 Open channels
- 154.44 Construction and materials

Stormwater Detention

- 154.55 General design
- 154.56 Acceptable methods
- 154.58 Allowable release rate
- 154.59 Drainage system overflow design
- 154.60 Determination of storage volume; methods
- 154.61 Basin requirements
- 154.62 Parking lot storage
- 154.63 Facility financial responsibility

- 154.64 Facility maintenance responsibility
- 154.65 Inspections
- 154.66 Corrective measures
- 154.67 Joint development
- 154.68 Installations
- 154.69 Detention facilities in floodplains
- 154.70 Off-site drainage

Plans and Determinations

- 154.80 Certifications required
- 154.81 Changes in plan
- 154.82 Determination of impact drainage areas
- 154.83 Sump pumps
- 154.84 Downspouts
- 154.85 Footing drains
- 154.86 Basement floor drains
- Appendix: Rainfall data

GENERAL PROVISIONS

§154.01 AUTHORITY AND JURISDICTION

This chapter is adopted pursuant to the authority of I.C. §36-9-28.5 and §36-1-3. The rules and regulations contained in this Chapter apply to all unincorporated areas in Montgomery County, Indiana and in Towns in Montgomery County that elect to allow this policy to apply inside the corporate boundaries.

(Ord. 90.5, passed 8-30-90; Am. Ord. passed 2-16-99; Am. Ord 2016-17, passed 6-27-16; Ord 2020- , passed May 11, 2020)

§154.02 PURPOSE

(A) (1) It is recognized that smaller streams and drainage channels serving the county may not have sufficient capacity to receive any convey stormwater runoff, resulting when land-use changes from open or agricultural use to a more urbanized use.

(2) It is further recognized that deposits of sediment from developments during and after construction can reduce capacities of storm sewers and drainage systems and result in damages.

(B) Therefore, it shall be the policy of the Drainage Board that the storage and controlled release of stormwater runoff shall be required of all new development, any redevelopment

and other new construction in the county. The release rate of stormwater from developed lands shall not exceed the release rate from the land area in its present land use.

(C) Because topography and the availability and adequacy of outlets for storm runoff vary with almost every site, the requirements for storm drainage tend to be an individual matter for any project. It is recommended that each proposed project be discussed with the County Surveyor's Office at the earliest practical time in the planning stage.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.05 DEFINITIONS

For the purpose of this chapter, the following definitions shall apply unless the context clearly indicates or requires a different meaning:

BOARD. The Drainage Board of the county and any subordinate employee to whom they shall specifically delegate a responsibility authorized by this chapter.

CAPACITY OF A STORM DRAINAGE FACILITY. The maximum flow that can be conveyed or stored by a storm drainage facility without causing damage to public or private property.

CHANNEL. A natural or artificial watercourse which periodically or continuously contains moving water or which forms a connecting link between two bodies of water. It has a defined bed and banks which serve to confine the water.

COMMISSION. The Montgomery County Plan Commission.

COMPENSATORY STORAGE. An artificial volume of storage within a floodplain used to balance the loss of natural flood storage capacity when artificial fill or structures are placed within the floodplain.

CONTIGUOUS. Adjoining or in actual contact with.

CULVERT. A closed conduit used for the passage of surface drainage water under a roadway, railroad, canal, or other impediment.

DETENTION BASIN.

(1) A facility constructed or modified to restrict the flow of stormwater to a prescribed maximum rate, and to detain concurrently the excess waters that accumulate behind the outlet.

(2) A basin designed to be completely dewatered after having provided its planned detention of runoff during a storm event.

DETENTION STORAGE. The temporary detaining or storage of stormwater in storage basins in streets, parking lots, schoolyards, parks, open spaces or other areas under predetermined and controlled conditions, with the rate of drainage therefrom regulated by appropriately installed devices.

DRAINAGE AREA. The area from which water is carried off by a drainage system, a watershed or catchment area.

DROP MANHOLE. A manhole having a vertical drop pipe connecting the inlet pipe to the outlet pipe. The vertical drop pipe shall be located immediately outside the manhole.

DURATION. The time period of a rainfall event.

EROSION. Wearing away of the land by running water, waves, temperature changes, ice or wind.

FLOOD ELEVATION. The elevation at all locations delineating the maximum level of high waters for a flood of given return period and rainfall duration.

FLOOD or FLOOD WATERS. The water of any watercourse which is above the banks of the watercourse. It also means the water of any lake which is above and outside the banks thereof.

FLOOD HAZARD AREA. Any floodplain, floodway, floodway fringe or any compilation thereof which is subject to inundation by the regulatory flood or any floodplain as delineated by Zone A on a flood hazard boundary map.

FLOODPLAIN. The area adjoining the river or stream which has been or may hereafter be covered by floodwaters.

FLOOD PROTECTION GRADE. The elevation of the lowest floor of a building. If a basement is included, the basement floor is considered the lowest floor.

FLOODWAY. See ***REGULATORY FLOODWAY.***

FLOODWAY FRINGE. The portion of the floodplain lying outside the floodway, which is inundated by the regulatory flood.

FOOTING DRAIN. A drain pipe installed around the exterior of a basement wall foundation to relieve water pressure caused by high groundwater elevation.

GRADE. The inclination or slope of a channel, canal, conduit and the like or natural ground surface usually expressed in terms of the percentage the vertical rise or fall bears to the corresponding horizontal distance.

IMPACT AREAS. Areas defined and mapped by the Board which are unlikely to be easily drained because of one or more factors including but not limited to any of the following: soil type, topography, land where there is not adequate outlet, a floodway or floodplain, land within 75 feet of each bank of any regulated drain or within 75 feet from the centerline of any regulated ditch.

IMPERVIOUS. A term applied to material through which water cannot pass or through which water passes with difficulty.

INLET. An opening into a storm sewer system for the entrance of surface stormwater runoff, more completely described as a storm sewer inlet.

JUNCTION CHAMBER. A converging section of conduit, usually large enough for a person to enter, used to facilitate the flow from one or more conduits into a main conduit.

LATERAL STORM SEWER. A sewer that has inlets connected to it but has no other storm sewer connected.

MANHOLE. Storm sewer structure through which a person may enter to gain access to an underground storm sewer or enclosed structure.

MAJOR DRAINAGE SYSTEM. Drainage system carrying runoff from an area of one or more square miles.

MINOR DRAINAGE SYSTEMS. Drainage systems having an area of less than one or more square miles.

OFF-SITE. Everything not on site.

ON-SITE. Located within the controlled area where runoff originates.

OUTFALL. The point or location where storm runoff discharges from a sewer or drain. Also applies to the outfall sewer or channel which carries the storm runoff to the point of outfall.

PEAK FLOW. The maximum rate of flow of water at a given point in a channel or conduit resulting from a particular storm or flood.

RADIUS OR CURVATURE. Length of radius of a circle used to define a curve.

RAINFALL INTENSITY. The cumulative depth of rainfall occurring over a given duration, normally expressed in inches per hour.

REACH. Any length of river, channel or storm sewer.

REGULATED AREA. All of the land under the jurisdiction of the Drainage Board. Provided however the provisions of this chapter shall not apply to the City of Crawfordsville and its contiguous two-mile jurisdictional limit surrounding the city.

REGULATORY FLOOD. That flood having a peak discharge which can be equaled or exceeded on the average of once in a 100-year period, as calculated by a method and procedure which is acceptable to the Board. If a permit from the National Resource Commission for construction in the floodway is required, then the regulatory flood peak discharge should be calculated by a method acceptable to the Board and the Natural Resources Commission. The REGULATORY FLOOD is equivalent to a flood having a probability of occurrence of 1% in any given year.

REGULATORY FLOODWAY. The channel of a river or stream and those portions of the floodplains adjoining the channel which are reasonably required to carry and discharge the peak flow of the regulatory flood of any river or stream.

RELEASE RATE. The amount of stormwater released from a stormwater control facility per unit of time.

RETENTION BASIN. A basin designed to retain a permanent pool of water after having provided its planned determination of runoff during a storm event.

RETURN PERIOD. The average interval of time within which a given rainfall event will be equaled or exceeded once. A flood having a RETURN PERIOD of 100 years has a 1% probability of being equaled or exceeded in any one year.

RUNOFF COEFFICIENT. A decimal fraction relating the amount of rain which appears as runoff and reaches the storm drainage system to the total amount of rain falling. A coefficient of 0.5 implies that 50% of the rain falling on a given surface appears as stormwater runoff.

SEDIMENT. Material of soil and rock origin, transported, carried or deposited by water.

SIPHON. A closed conduit or portion of which lies above the hydraulic grade line, resulting in a pressure less than atmospheric and requiring a vacuum within the conduit to start flow. A ***SIPHON*** utilizes atmospheric pressure to effect or increase the flow of water through a conduit. An ***INVERTED SIPHON*** is used to carry stormwater flow under an obstruction such as a sanitary sewer.

SPILLWAY. A waterway in or about a hydraulic structure for the escape of excess water.

STILLING BASIN. A basin used to slow water down or dissipate its energy.

STORAGE DURATION. The length of time that water may be stored in any stormwater control facility, computed from the time water first begins to be stored.

STORM SEWER. A closed conduit for conveying collected stormwater.

STORMWATER DRAINAGE SYSTEM. All means, natural or manmade, used for conducting stormwater to, through or from a drainage area to any of the following: conduits and appurtenant features, canals, channels, ditches, stream, culverts, streets and pumping stations.

STORMWATER RUNOFF. The water derived from rains falling within a tributary basin, flowing over the surface of the ground or collected in channels or conduits.

TRIBUTARY. Contributing stormwater from upstream land areas.

URBANIZATION. The development, change or improvement of any parcel, of land consisting of one or more lots for residential, commercial, industrial, institutional, recreational or public utility purposes.

WATERCOURSE. Any river, stream, creek, brook, branch, natural or manmade drainageway in or into which stormwater runoff or floodwaters flow either regularly or intermittently.

WATERSHED. See ***DRAINAGE AREA.***
(Ord. 90-5, passed 8-30-90; Am. Ord. passed 9-25-01)

§154.06 DISCLAIMER OF LIABILITY

(A) The degree of protection required by this chapter is considered reasonable for regulatory purposes and is based on historical records, engineering and scientific methods of study.

(B) Larger storms may occur or stormwater runoff depths may be increased by manmade or natural causes. This chapter does not imply that land used permitted will be free from stormwater damage. This chapter shall not create liability on the part of the county or any officer or employee thereof for any damage which may result from reliance on this chapter or on any administrative decisions lawfully made thereunder. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.07 VIOLATIONS

(A) Continuing Violations: A separate offense shall be deemed to occur for each day a violation of this chapter continues to exist.

(B) Notice of Violation. The County Engineer shall inform the owner in writing when a violation has been found. The notice to correct will include a description of the violation, the action necessary to be taken by the violator in order to correct the violation, the reasonable time to be allowed for the correction of the violation, the amount of fines per day which will accrue and be owed by the violator if the violation is not corrected, and the procedure available to the violator to appeal the determination of the County Engineer that a violation exists.

(C) Appeal of Notice of Violation. The person or entity receiving a notice of violation has the right to appeal the County Engineer's determination to the Plan Commission. In order to appeal the determination, the person must file a written appeal with the Secretary of the Commission a statement which states all facts which supports the appeal. This statement must be mailed or delivered to the Secretary within ten (10) days of the receipt of the notice of violation. If the person or entity receiving the notice of violation fails to file a timely appeal, the person or entity is deemed to have waived the right to appeal and to have admitted the violation.

(D) Hearing on Appeal. Upon receipt of a timely filed appeal, the Secretary of the Commission will notify the Board President of such receipt, and the President will schedule a public hearing on the appeal and conduct the appeal hearing within 30 days of the receipt of the timely filed appeal. The Secretary will provide written notice of the hearing to the party filing the appeal and to the County Engineer. At the hearing, the party appealing the notice of violation may be represented by an attorney, may present evidence which supports the appeal, and may present any legal authority in support of the appeal. After the party filing the appeal concludes this presentation, the County Engineer will be afforded the opportunity to respond to the appeal and present evidence and authority supporting the County Engineer's determination that a violation exists or existed at the time of the issuance of the notice. The Commission will make findings and conclusions supporting its decision on the appeal within 10 days of the hearing, and the Secretary of the Commission will mail copies of its decision to the party appealing the determination and to the County Engineer.

(E) Appeal of Commission: The party filing the appeal may appeal the decision of the Commission by filing with a court of competent jurisdiction a petition for review within 30 days of the party's receipt of the Commission's decision.

(F) Fine Citation. If the person receiving the notice of violation does not correct the violation, the County Engineer will mail to the violator a fine citation which states the fine due from the violator. The fine is payable to Montgomery County and is due within 30 days of the date of the citation.

(G) Fines. For each violation of this Chapter, the violator is subject to fine in the amount of \$100 per day.

(H) Enforcement. The County Engineer will have the right to file a case for the collection of any and all unpaid fines. In addition, the County Engineer will have the right to issue a stop-work order, obtain injunctive relief in order to prevent the continuing violation of this Chapter, and to obtain any and all other legal relief provided for by Indiana law in order to enforce this Chapter. In addition, in the event that the County Engineer incurs expenses for court costs and/or attorney's fees for the enforcement of this Chapter, the violator will be liable for such court costs and/or attorney's fees.

(I) Other Remedies. Nothing herein shall prevent the county from taking other lawful action to prevent or remedy any violations. All costs connected therewith shall be charged to the person or persons responsible.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.08 RESERVED

§154.09 FEES

(A) All plans for development, construction, extensions, additions, parking lots, ponds, dams, levees and other improvements must be evaluated for stormwater impact and approved by the County Engineer prior to the issuance of a location improvement permit.

(B) The following user fees shall take effect upon adoption and publication of notice of the adoption in accordance with the law:

(1) Site trip review to see if the impact will require that a plan be submitted, (dimensioned site plan showing existing structures, streams, drainage tile, well and septic and the proposed changes is required): \$40.

(2) Additional trips required because of changes or incomplete information: \$30.

(3) Plan reviews for plans submitted for approval by the Commission: The amount charged to the Commission or County Engineer by a professional engineer to review or, if reviewed by the County Engineer the amount equal to the time required for the review multiplied by \$100 per hour.

(C) All fees must be paid in full before a location improvement permit can be issued.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

DRAINAGE CONTROL POLICY

§154.20 POLICY

(A) It is recognized that the smaller streams and drainage channels serving the county may not have sufficient capacity to receive and convey stormwater runoff resulting from continued urbanization. Accordingly, the storage and controlled release rate of excess stormwater runoff shall be required for any development, redevelopment and new construction located within the county.

(B) The Commission, after thorough investigation and evaluation, may waive the requirement of controlled runoff for minor subdivisions and parcelization.

(C) The release rate of stormwater from developments, redevelopments, and new construction may not exceed the stormwater runoff from the land area in its present state of development. The developer must submit to the Commission, detailed computations of runoff before and after development, redevelopment or new construction which demonstrate that runoff will not be increased.

(D) These computations must show that the peak runoff rate after development for the 100-year return period storm of critical duration must not exceed the three-year return period predevelopment peak runoff rate. The critical duration storm is that storm duration that requires the greatest detention storage.

(E) Computations for project areas of ten (10) acres or more will be based upon the National Resources Conservation Service (NRCS) Curve Number Method for determining runoff rate and sizing storm water storage. Computations for areas of less than ten (10) acres will use the Rational Method for determining runoff rate and sizing storm water storage. Project area mean the entire area of the parcel (not just disturbed area). For areas larger than 200 acres, hydrograph techniques and/or computer drainage modeling methods may be used.

(F) Hydrograph techniques and computer modeling methods used to determine stormwater runoff shall be proven methods, subject to approval of the Board.
(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.21 PERMITS FOR CONSTRUCTION

(A) Chapter 318 of the Acts of 1945, as amended, Sections 17 and 19, require the Natural Resources Commission approval of any construction in a floodway, and of any works for flood control. This includes bridges, dams, levees, dikes, floodwalls, wharves, piers, dolphins, booms, weirs, bulkheads, jetties, groins, excavations, fills or deposits of any kind, utility lines or any other building, structure or obstruction.

(B) Also, any ditch work (new construction, deepening or modification) within one-half mile of a public freshwater lake of ten acres or more in area.

(C) The approval of the Natural Resources Commission, in writing, must be obtained before beginning construction.

(1) Applications for approval should be submitted to the Department of Natural Resources, Division of Water, 2475 Directors Row, Indianapolis, Indiana 46241.

(2) All applications should be made on the standard application form provided by the Commission and should be accompanied by plans, profiles, specifications and other data necessary for the Commission to determine the effect of the proposed construction upon the floodway and on flood control in the state.

(3) Application made to and approved granted by the Natural Resources Commission does not in any way relieve the owner of the necessity of security easements or other property rights, and permits and/or approvals from affected property owners and local, state and federal agencies.

(D) The engineering staff of the Division of Water is available to discuss and offer suggestions regarding requirement sin the design of structures in floodways. High water marks have been set on many of the streams in the state, and information is available from the Division of Water on actual and/or potential flooding. Information regarding bench marks set to Mean Sea Level Datum, General Adjustment of 1929, is available from the Division of Water, Surveying and Mapping Section.

(E) Applications are considered by the Commission at regular meetings usually held each month. After the application and plans have been approved by the Commission, a certificate of approval is forwarded to the applicant.

(F) A fee is charged by the Commission for approvals under the Flood Control Act. Unless stated otherwise in the approval, construction is considered to be a permanent development and no renewals of the approval are necessary, except in the cases where temporary approvals are granted for temporary construction. The right is reserved to require additional data where necessary.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.22 INFORMATION REQUIREMENTS

(A) *Submission Required.* As provided in divisions (B) through (G), the information and data provided by a state-licensed professional engineer or land surveyor engaged in storm drainage design shall be submitted to the Building Commissioner at the time of application for a building permit for any development, redevelopment or new construction on real estate which lies within the regulated area.

(B) *Topographic and Soils Map.* A soils map of the proposed development indicating soils names and their hydrologic classification must be provided when Soil Conservation Service (SCS) hydrologic methods are used. In addition, a topographic map of the land to be subdivided and adjoining land whose topography may affect the layout or drainage of the development must be provided. The contour intervals shall be one foot when slopes are less than 4% and shall be two feet when the slope exceeds 4%. On this map, the following shall be shown:

- (1) The location of streams and other floodwater runoff channels, the extent of the floodplains at the established 100-year flood elevation where available, regulatory floodway and the limits of the floodway, all properly identified;
- (2) The normal shoreline of lakes, ponds, swamps and detention basins, their floodplains and lines of inflow and outflow, if any;
- (3) The location of regulated drains, farm drains, inlets and outfalls, if any of record;
- (4) Storms, sanitary and combined sewers and outfalls, if any of record;
- (5) Septic tank systems and outlets, if any of record, or as otherwise known to applicant; and
- (6) Seeps, springs, flowing and other wells that are visible or of record.

(C) *Preliminary Drainage Plan.* A comprehensive plan, in preliminary form (or in combined preliminary and final form), designed to handle safely the stormwater runoff and to detain the increased stormwater runoff must be provided. The plan shall provide or be accompanied by maps or other descriptive materials indicating the feasibility of the drainage plan and showing the following:

- (1) The extent and area of each watershed affecting the design of detention facilities as shown on USGS quadrangle maps or other more detailed maps as required by the Building Commissioner, County Engineer or Commission and provide a vicinity map which geographically locates project area within the county;
- (2) The preliminary layout and design of proposed storm sewers, the outfall and outlet locations and approximate elevations, the receiving stream or channel and its 100-year return period water elevation;
- (3) The location and design of proposed street systems used to convey or temporarily store overflow from the heavier rainstorms and the outlets for the overflow;
- (4) The locations, cross-sections and profiles of existing streams and floodplains to be

maintained and new channels to be constructed;

(5) The materials, elevations, waterway openings and the basis for design of proposed culverts and bridges;

(6) Existing detention ponds and basins to be maintained, enlarged or otherwise altered and new ponds or basins to be built and the basis of their design;

(7) The estimated depth and amount of storage required in the new ponds or basins;

(8) The estimated location and percentage of impervious surfaces existing and expected to be constructed when the development is completed; and

(9) Any interim plan which is to be incorporated into the development pending completion of the development and the final drainage plan.

(D) *Valley Cross Section.* One or more typical cross-sections must be provided showing all existing and proposed channels or other open drainage facilities carried to a point above the 100-year high water elevation showing the elevation of the existing land and the proposed changes thereto, together with the high water elevations expected from the 100-year storm under the controlled conditions called for by this chapter and showing the relationship of structures, streets and other facilities.

(E) *Site Plan.* A plan drawn to scale showing dimensions of the site with existing and proposed storm drainage facilities must be provided.

(F) *Final Drainage Plans.* Upon approval of the preliminary drainage plans by the Commission, final drainage plans shall be submitted to the Commission. The final plans shall provide or be accompanied by calculations, maps and/or other descriptive material showing the following:

(1) The extent and area of each watershed tributary to the drainage channels in the development;

(2) The street storm sewers and other storm drains to be built, the basis of their design, outfall and outlet locations and elevations, the receiving stream or channel and its high-water elevation, and the functioning of the drains during high water conditions;

(3) The parts of the proposed street system where pavements are planned to be depressed sufficiently to convey or temporarily store overflow from storm sewers and over the curb runoff resulting from the heavier rainstorms and the outlets for the overflow;

(4) Existing streams and floodplains to be maintained and new channels to be

constructed their locations, cross-section and profiles;

(5) Proposed culverts and bridges to be built, their materials, elevations, waterway openings and basis of their design;

(6) Existing detention basins and ponds to be maintained, enlarged or otherwise altered and new basins or ponds to be built and the basis of their design;

(7) The estimated location and percentage of impervious surfaces existing and expected to be constructed when the development is completed;

(8) The slope, type and size of all sewers and other waterways;

(9) For all detention basins, a plot or tabulation of storage volumes with corresponding water surface elevations and a plot or tabulation of the basin outflow rates for those water surface elevations.

(10) In all platted residential developments, the owner will provide for a twenty-foot (20') drainage easement along the subdivision perimeter and along the rear yards of all lots which are not along the perimeter. These easements will be shown on the plans.

(G) *Submittal; Consideration of Plans.*

(1) Preliminary and final drainage plans and/or construction plans shall be submitted to the Commission 20 days prior to their regularly scheduled meeting. All preliminary plans, final plans and/or construction plans in compliance with the standards of this chapter shall be approved by the Commission. The Commission and/or the County Engineer shall stamp the approval on a copy of the plans and deliver the same to the applicant. The Commission shall approve or disapprove any preliminary plans, final plans and/or construction plans within 60 days of submission unless the applicant consents to a continuance or extensions.

(2) All approvals and disapprovals with written reasons shall be incorporated into the Commission minutes. The County Engineer is authorized to review engineering summaries of projects and based upon the same grant exemptions from any and all requirements of this chapter and/or waive any requirements of this chapter. Any applicant may appeal the decision of the Engineer to the Commission which shall also be authorized to grant exemptions from any and all requirements of this chapter and/or waive any requirements of this chapter at its discretion. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99).

§154.23 DETERMINATION OF RUNOFF QUANTITIES

(A) Runoff quantities shall be computed for the area of the parcel under development plus the area of the watershed flowing into the parcel under development.

(B) The quantity of runoff which is generated as the result of a given rainfall intensity may be calculated as follows:

(1) (a) For areas up to and including 200 acres, the rational method may be used. In the rational method, the peak rate of runoff, Q, in cubic feet per second is computed as:

$$Q = CIA$$

(i) C equals runoff coefficient, representing the characteristics of the drainage area and defined as the ratio of runoff to rainfall.

(ii) I equals average intensity of rainfall in inches per hour for a duration equal to the time of concentration (tc) for a selected rainfall frequency.

(iii) A equals tributary drainage area in acres. Guidance to selection of the runoff coefficient "C" is provided by the first table below, which show values for different types of surface and local soil characteristics.

(b) The composite "C" value used for a given drainage area with various surface types shall be the weighted average value for the total area calculated from a breakdown of individual areas having different surface types.

(c) The second table below provides runoff coefficients and inlet times for different land use classifications. In the instance of undeveloped land situated in an upstream area, a coefficient or coefficients shall be used for this area in its present or existing state of developments.

Urban Runoff	
Type of Surface	Runoff Coefficient "C"
Asphalt	0.82
Concrete	0.85
Roof	0.85
Lawns (Sandy):	
Flat (0-2%)	0.07
Rolling (2-7%)	0.12
Steep (greater than 7%)	0.17
Lawns (Clay):	
Flat (0-2%)	0.16
Rolling (2-7%)	0.21
Steep (greater than 7%)	0.30
Woodland (Sandy):	
Flat (0-5% Slope)	0.10
Rolling (5-10% Slope)	0.25
Steep (greater than 10%)	0.30
Woodland (Clay):	
Flat	0.30
Rolling	0.35
Steep	0.50
Pasture (Sandy):	
Flat	0.10
Rolling	0.16
Steep	0.22
Urban Runoff	
Type of Surface	Runoff Coefficient "C"
Pasture (Clay):	
Flat	0.30
Rolling	0.36
Steep	0.42
Cultivated (Sandy):	
Flat	0.30
Rolling	0.40
Steep	0.52
Cultivated (Clay):	
Flat	0.50
Rolling	0.60
Steep	0.72

NOTES TO TABLE:

The coefficients of this tabulation are applicable to storms of five- to ten-year frequencies. Coefficients for less frequent higher intensity storms shall be modified as follows:

<u>Return Period (years)</u>	<u>Multiply "C" by</u>
25	1.1
50	1.2
100	1.25

Land Use and Typical Inlet Times

<i>Land Use</i>	<i>Runoff Coefficients</i>			<i>Inlet Times (minutes)</i>
	<i>Flat</i>	<i>Rolling</i>	<i>Steep</i>	
Commercial (CBD)	0.75	0.83	0.91	5
Commercial (Neighborhood)	0.54	0.60	0.66	5 - 10
Industrial	0.63	0.70	0.77	
Garden Apartments	0.54	0.60	0.66	
Churches	0.54	0.60	0.66	
Schools	0.31	0.35	0.39	10 - 15
Semi-Detached Residential	0.45	0.50	0.55	
Quarter-Acre Lots	0.36	0.40	0.44	
Half-Acre Lots	0.31	0.35	0.39	
Parkland	0.18	0.20	0.22	TBC

NOTES TO TABLE:

Flat terrain 0-2% slopes.

Rolling terrain 2-7% slopes.

Steep terrain greater than 7% slopes.

Interpolation, extrapolation and adjustment for local conditions shall be based on engineering experience and judgment.

The coefficients of this tabulation are applicable to storms of five- to ten-year frequencies.

Coefficients for less frequent higher intensity storms shall be modified as follows:

<u>Return Period</u>	<u>Multiply "C" by</u>
25	1.1
50	1.2
100	1.25

(d) Rainfall intensity shall be determined from the rainfall frequency curves shown in § 1 of the appendix to this chapter or from data shown in § 154.58(B). The time of concentration (tc) to be used shall be the sum of the inlet time and flow time in the drainage facility from the most remote part of the drainage area to the point under consideration. The flow time in the storm sewers may be estimated by the distance in feet divided by velocity of flow in feet per second. The velocity shall be determined by the Manning Formula. Inlet time is the combined time required for the runoff to reach the inlet of the storm sewer. It includes overland flow time and flow time through established surface drainage channels such as swales, ditches and sheet flow across such areas as lawns, fields and other graded surfaces. It may be computed by using § 2 of the appendix to this chapter.

(2) The runoff rate for areas in excess of 200 acres shall be determined by methods described herein.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.24 AMOUNT TO BE ACCOMMODATED

Various parts of drainage facility must accommodate runoff water as follows:

(A) The minor drainage system such as inlets, catch basins, street gutters, swales, sewers and small channels which collect stormwater must accommodate peak runoff from a three-year return period storm. Rainfall duration shall be equal to the time of concentration or one hour if the time of concentration is less than one hour. A first quartile storm distribution shall be used for computer modeling. The following additional requirements must be satisfied:

(1) Open channels carrying peak flows greater than 30 cubic feet per second shall be capable of accommodating peak runoff for a 50-year return period storm within the drainage easement.

(2) Culverts shall be capable of accommodating peak runoff from a 50-year return period storm when crossing under a road which is part of the State Department of Highways rural functional classification system and are classified as principal or minor arterial, major or minor collector roads.

(3) Temporary water storage will not be permitted on any local street or road, except by special exception of the Commission.

(4) In urban areas first floor elevations of all living units, commercially or industrially used buildings, shall be such that all floors including basements shall two feet of freeboard above the 100-year flood elevation or at the, flood protection grade. In urban areas the land grade at houses shall be based upon the maximum flood of record or upon a flood which may occur once in 100 years, whichever is greater, together with a freeboard of two to three feet.

(B) Major drainage systems are defined herein and shall be designed in accordance with State Department of Natural Resources Standards, as described herein.
(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

DESIGN STANDARDS

§154.35 GENERAL DESIGN

All storm sewers subject to this chapter, whether private or public and whether constructed on private or public property, shall conform to the design standards and other requirements contained herein.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.36 MANNING EQUATION

(A) The hydraulic capacity of storm sewers shall be determined using Manning's Equations:

$$V = \frac{1.486}{n} R^{2/3} S^{1/2}$$

(1) V equals mean velocity of flow in feet per second.

(2) R equals the hydraulic radius in feet.

(3) S equals the slope of the energy grade line in feet per foot.

(4) n equals roughness coefficient.

(B) The hydraulic radius, **R**, is defined as the cross-sectional area of flow divided by the wetted flow surface or wetted perimeter. Typical “n” values and maximum permissible velocities for storm sewer materials are listed herein. Roughness coefficient “n” values for other sewer materials can be found in standard hydraulics texts and references. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.37 MINIMIZE SIZE

The minimum size of all storm sewers shall be 12 inches. The rate of release for detention storage shall be controlled by an orifice plate or other devices, subject to approval of the Board, where the 12-inch pipe will not limit rate of release as required. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.38 GRADE

(A) Sewer grade shall be such that, in general, a minimum of two feet of cover is maintained over the top of the pipe.

(B) Pipe cover less than the minimum may be used only upon approval of the Board. Uniform slopes shall be maintained between inlets, manholes and inlets to manholes.

<i>Typical Values of Manning's</i>		
<i>Material</i>	<i>Manning's</i>	<i>Desirable Maximum Velocities</i>
Closed Conduits:		
Concrete	0.013	15 f.p.s.
Vitrified	0.013	15 f.p.s.
Brick	0.015	15 f.p.s.
Cast Iron	0.013	15 f.p.s.
Circular Corrugated Metal Pipe, Annular Corrugations 2 2/3 x 1/2 inches:		
Unpaved	0.024	7 f.p.s.
25% paved	0.021	7 f.p.s.
50% paved	0.018	7 f.p.s.
100% paved	0.013	7 f.p.s.
Circular Corrugated Metal Pipe, Helical, 2 2/3 x 1/2 inches; Unpaved Corrugations;		
12 inches	0.011	
18 inches	0.013	
24 inches	0.015	
36 inches	0.018	

48 inches	0.020	
60 inches or larger	0.021	
Corrugated polyethylene smooth interior pipe	0.012	15 f.p.s.
Concrete culverts	0.013	
Open Channels:		
Concrete, trowel finish	0.013	
Concrete, broom or float finish	0.015	
Gunitite	0.018	
Riprap placed	0.030	
Riprap dumped	0.035	
Gabion	0.028	
New earth (uniform, sodded, clay)	0.025	
Existing earth (fairly uniform with some weeds)	0.030	
Dense growth of weeds	0.040	
Dense weeds and brush	0.040	
Swale with grass	0.035	

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.39 ALIGNMENT

(A) Storm sewers shall be straight between manholes insofar as possible. Where long radius curves are necessary to conform to street layout, the minimum radius of curvature shall be no less than 100 feet for sewers 42 inches and larger in diameter.

(B)(1) Deflection of pipe sections shall not exceed the maximum deflection recommended by the pipe manufacturer.

(2) The deflection shall be uniform and finished installation shall follow a smooth curve. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.40 MANHOLES

(A) Manholes shall be installed to provide access to continuous underground storm sewers for the purpose of inspection and maintenance. Manholes shall be provided at the following locations:

(1) Where two or more storm sewers converge;

(2) At the point of beginning or at the end of a curve and at the point of reverse curvature;

- (3) Where pipe size changes;
- (4) Where an abrupt change in alignment occurs;
- (5) Where a change in grade occurs; and/or
- (6) At suitable intervals in straight sections of sewer.

(B) The maximum distance between storm sewer manholes sections of sewer.

<i>Size of Pipe</i>	<i>Maximum Distance</i>
12 inches through 42 inches	400 feet
48 inches and larger	600 feet

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.41 INLETS

(A)(1) Inlets or drainage structures shall be utilized to collect surface water through grated openings and convey it to storm sewers, channels or culverts.

(2) Inlet design and spacing shall be in accordance with § 7-400 of the State Department of Highways' Road Design Manual Volume 1 or other approved design procedure. The inlet grate openings provided must be adequate to pass the design three-year flow with 50% of the sag inlet areas clogged.

(B) An overload channel from sag inlets to the overflow channel or basin shall be provided at sag inlets, so that the maximum depth of water that might be ponded in the street sag shall not exceed six inches.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.42 WORKMANSHIP AND MATERIALS

(A) The specifications for the construction of storm sewers shall not be less stringent than those set forth in the latest edition of the State Department of Highways' Standard Specifications. Additionally, ductile iron pipe shall be laid in accordance with American Society of Testing Materials (ASTM) C-12.8.

(B) Storm sewer manholes and inlets shall be constructed of masonry, cast in place concrete or precast reinforced concrete. Material and construction shall conform to State Department of Highways' Standard Specifications, § 720. Pipe and fittings used in storm sewer

construction shall be extra-strength clay pipe (ASTM C-700), ductile iron pipe (AWWA C-151), or concrete pipe (ASTM C-76). Other pipe and fittings not specified herein may be used only when specifically authorized by the Commission. Pipe joints shall be flexible and watertight and shall conform to the requirements of § 715.02 of the latest edition of the State Department of Highways' Standard Specifications. All storm sewers within public rights-of-way will be concrete or corrugated metal pipe and able to handle HS-20 loading. The minimum size of a storm sewer is 12 inches.

(C) (1) Special hydraulic structures required to control the flow of water in storm runoff drainage systems include junction chambers, drop manholes, inverted siphons, stilling basins and other special structures.

(2) The use of these structures shall be limited to those locations justified by prudent planning and by careful and thorough hydraulic engineering analysis.
(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.43 OPEN CHANNELS

(A) All open channels subject to this chapter, whether private or public and whether constructed on private or public land, shall conform to the design standards and other design requirements contained herein.

(B) The waterway for channels shall be determined using Manning's Equation.

$$Q = AV = \underline{1.486} (2/3)(1/2) N(R)(S)$$

(1) A equals waterway area of channel in square feet.

(2) Q equals discharge in cubic feet per second (cfs).

(3) V, R, S and N are explained herein.

(C) The required channel cross-section and grade are determined by the design capacity, the material in which the channel is to be constructed, and the requirements for maintenance. A minimum depth may be required to provide adequate outlets for subsurface drains, tributary ditches or streams. The channel grade shall be such that the velocity in the channel is high enough to prevent siltation but low enough to prevent erosion. Velocities less than one and one-half feet per second should be avoided because siltation will take place and ultimately reduce the channel cross-section. The maximum permissible velocities in vegetated channels are shown herein. Developments through which the channel is to be constructed must be considered in design of the channel section. Earthen channel side slopes

shall be no steeper than two to one. Flatter slopes may be required to prevent erosion and for ease of maintenance.

(D) Where channels will be lined, side slopes shall be no steeper than one and one-half to one with adequate provision made for weep holes. Side slopes steeper than one and one-half to one may be used for lined channels provided that the sidelining and structural retaining wall are designed and constructed with provisions for live and dead load surcharge.

<i>Maximum Permissible Velocities in Vegetal-Lined Channels</i>			
<i>Cover</i>	<i>Slope Range (2%)</i>	<i>Permissible Velocity</i>	
		<i>Erosion Resistant Soils (feet per second)</i>	<i>Easily Eroded Soils (feet per Second)</i>
Bermuda grass	0 – 5	8	6
	5 – 10	7	5
	Over 10	6	4
Bahia Buffalo grass Kentucky bluegrass Smooth brome Bluegrass	0 – 5	7	5
	5 – 10	6	4
	Over 10	5	3
Grass mixtures Reed canary grass	0 – 5	5	4
	5 – 10	4	3

Lespedeza Sericea Weeping love grass Yellow bluestem Redtop Alfalfa Red fescue	0 – 5	3.4	2.5
Common Lespedeza(4) Sudangrass (4)	(5)		
	0 – 5	3.5	2.5
NOTES TO TABLE: Use velocities exceeding five feet per second only where good covers and proper maintenance can be obtained. Do not use on slopes steeper than 10%, except for vegetated side slopes in combination with stone, concrete or highly resistant vegetative center section. Do not use on slopes steeper than 5%, except for vegetated side slopes in combination with stone, concrete or highly resistant vegetative center section. Annuals use on mild slopes or as temporary protection until permanent covers are established. Use on slopes steeper than 5% is not recommended.			

(E) (1) Characteristics of a stable channel are:

- (a)** It neither aggrades nor degrades beyond tolerable limits.
- (b)** The channel banks do not erode to the extent that the channel cross-section is changed appreciably.
- (c)** Excessive sediment bars do not develop.
- (d)** Excessive erosion does not occur around culverts, bridges or elsewhere.
- (e)** Gullies do not form or enlarge due to the entry of uncontrolled surface flow to the channel.

(2) Channel stability shall be determined for an aged condition and the velocity shall be based on the design flow or the bank full flow, whichever is greater, using “n” values for various channel linings as shown herein. In no case is it necessary to check channel stability for discharges greater than that from a 100-year period storm.

(3) Channel stability must be checked for conditions immediately after construction. For this stability analysis, the velocity shall be calculated for the expected flow from a ten-year return period storm on the watershed, or the bank full flow, whichever is smaller. The "n" value for new constructed channels in fine-grained soils and sands may be determined in accordance with the National Engineering Handbook 5, Supplement 8, Soil Conservation Service and shall not exceed 0.025. The allowable velocity in the newly constructed channel may be increased by a maximum of 20% to reflect the effects of vegetation to be established under the following conditions:

(a) The soil and site in which the channel is to be constructed are suitable for rapid establishment and support of erosion controlling vegetation;

(b) Species of erosion controlling vegetation adapted to the area and proven methods of establishment are shown; and

(c) The channel design includes detailed plans for establishment of vegetation on the channel side slopes.

(F) Vegetated waterways that are subject to low flows of long duration or where wet conditions prevail shall be drained with a tile system or by other means such as paved gutters. The lines may be outlet through a drop structure at the end of the waterway or through a standard tile outlet.

(G) **(1)** When the Board determines it is necessary to establish a new regulated drain, each developer must provide the necessary information and meet the requirements of the 1965 State Drainage Code, as amended, for the establishment of a new regulated drain.

(2) The Board shall determine the necessary easements for adequate maintenance of any new regulated drain.

(H) **(1)** The design of channels will provide all structures required for the proper functioning of the channel and the laterals thereto and travel ways for operation and maintenance.

(2) Recessed inlets and structures needed for entry of surface and subsurface flow into floodway.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.44 CONSTRUCTION AND MATERIALS

(A) Specifications shall be in keeping with the current standards of engineering practice and shall describe the requirements for proper installation of the project to achieve its intended purpose.

(B) Materials acceptable for use as channel lining are:

- (1)** Grass;
- (2)** Revetment riprap;
- (3)** Concrete;
- (4)** Hand-laid riprap;
- (5)** Precast cement concrete riprap;
- (6)** Grouted riprap; and
- (7)** Gabions.

(C) **(1)** Spoil material resulting from clearing, grubbing and channel excavation shall be disposed in a manner which will:

- (a)** Minimize over bank wash;
 - (b)** Provide for the free flow of water between the channel and floodplain unless the valley routing and water surface profile are based on continuous dikes being installed;
 - (c)** Not hinder the development of travel ways for maintenance;
 - (d)** Leave the right-of-way in the best condition feasible, consistent with the project purposes, for productive use by the owner;
 - (e)** Improve the aesthetic appearance of the site to the extent feasible; and
 - (f)** Be approved by the IDNR or US Army Corps of Engineers (whichever is applicable) if deposited in the channels without significant erosion or degradation shall be included in the design of channel improvements. The design is also to provide the necessary flood gates, water level control devices and any other appurtenance affecting the functioning of the channels and the attainment of the purpose for which they are built.
- (2)** The effect of channel improvements on existing culverts, bridges, buried cables, pipelines and inlet structures for surface and subsurface drainage on the channel being improved and laterals thereto shall be evaluated to determine the need for modification or replacement. Culverts and bridges which are modified or added as part of channel improvement projects shall meet reasonable standards for the type of structure and shall

have a minimum capacity equal to the design discharge or governmental agency design requirements, whichever is greater.

(3) Other lining materials shall receive specific approval of the Commission or Board. Materials shall comply with the latest edition of the State Department of Highways' Standard Specifications. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99).

STORMWATER DETENTION

§154.55 GENERAL DESIGN

The following shall govern the design of any improvement with respect to the detention of stormwater runoff.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.56. ACCEPTABLE METHODS

The increased stormwater runoff resulting from a proposed development shall be detained onsite by the provisions of appropriate detention or retention basins, parking lots, lawns or other acceptable techniques. Measures which further retard the rate of overland flow and the velocity in runoff channels may also be required to partially control the runoff rate. Detention basins shall be sized to store excess flows from storm with a 100-year return period. Control devices shall limit the discharge to a rate not greater than that prescribed by this chapter. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.58 ALLOWABLE RELEASE RATE

(A) The allowable release rate of stormwater originating from a proposed development shall not exceed the amount specified herein. In the event the natural downstream channel or storm sewer system is inadequate to accommodate the release rate provided above, then the allowable release rate shall be reduced to that rate permitted by the capacity of the receiving downstream channel or storm sewer system and additional detention as determined by the Board shall be required to store that portion of the runoff exceeding the capacity of the receiving sewers or waterways.

(B) If more than one detention basin is involved in the development of the area upstream of the limiting restriction, the allowable release rate from any one detention basis shall be in direct proportion to the ration of its drainage area to the drainage area of the entire watershed upstream of the restriction.

<i>Rainfall Depths (inches)</i>						
<i>Duration</i>	<i>Return Period (years)</i>					
	2	5	10	25	50	100
5 min.	0.42	0.52	0.59	0.68	0.75	0.82
10 min.	0.64	0.79	0.91	1.04	1.15	1.25
15 min.	0.80	0.99	1.13	1.29	1.43	1.55
20 min.	0.95	1.17	1.34	1.53	1.70	1.85
30 min.	1.11	1.34	1.57	1.79	1.98	2.16
40 min.	1.23	1.52	1.74	1.99	2.20	2.40
50 min.	1.33	1.64	1.87	2.14	2.36	2.58
60 min.	1.40	1.73	1.97	2.26	2.49	2.72
1.5 hrs.	1.70	2.09	2.39	2.73	3.03	3.30
2 hrs.	1.71	2.11	2.41	2.76	3.05	3.33
3 hrs.	1.84	2.27	2.60	2.97	3.29	3.59
4 hrs.	2.06	2.54	2.90	3.31	3.67	4.00
5 hrs.	2.16	2.66	3.04	3.48	3.85	4.20
6 hrs.	2.22	2.74	3.13	3.58	3.96	4.32
7 hrs.	2.31	2.85	3.26	3.72	4.12	4.49
8 hrs.	2.35	2.90	3.32	3.79	4.20	4.58
9 hrs.	2.42	2.99	3.41	3.90	4.32	4.71
10 hrs.	2.49	3.07	3.51	4.01	4.44	4.84
12 hrs.	2.58	3.18	3.63	4.15	4.59	5.01
14 hrs.	2.66	3.29	3.76	4.30	4.75	5.18
16 hrs.	2.75	3.40	3.68	4.44	4.91	5.39
18 hrs.	2.82	3.48	3.98	4.55	5.03	5.49
20 hrs.	2.89	3.56	4.07	4.65	5.15	5.62
24 hrs.	3.00	3.70	4.23	4.83	5.35	5.83

<i>Rainfall Intensity (inches/hour)</i>						
<i>Duration</i>	<i>Return Period (years)</i>					
	2	5	50	25	50	100
5 min.	5.04	6.24	7.08	8.16	9.00	9.84
10 min.	3.84	4.74	5.46	6.24	6.90	7.50
15 min.	3.20	3.96	4.52	5.16	5.72	6.20
20 min.	2.85	3.51	4.02	4.59	5.10	5.55
30 min.	2.22	2.74	3.12	3.58	3.96	4.32
40 min.	1.85	2.28	2.61	2.99	3.30	3.60
50 min.	1.60	1.97	2.24	2.57	2.83	3.10
60 min.	1.40	1.73	1.97	2.25	2.49	2.72
1.5 hrs.	1.13	1.39	1.59	1.82	2.02	2.20
2 hrs.	0.86	1.06	1.21	1.38	1.53	1.67
3 hrs.	0.61	0.76	0.87	0.99	1.10	1.20
4 hrs.	0.52	0.64	0.73	0.83	0.92	1.00
5 hrs.	0.43	0.53	0.61	0.70	0.77	0.84
6 hrs.	0.37	0.46	0.52	0.60	0.66	0.72
7 hrs.	0.33	0.41	0.47	0.53	0.59	0.64
8 hrs.	0.29	0.36	0.42	0.47	0.53	0.57
9 hrs.	0.27	0.33	0.38	0.43	0.48	0.52
10 hrs.	0.25	0.31	0.35	0.40	0.44	0.48
12 hrs.	0.22	0.27	0.30	0.35	0.38	0.42
14 hrs.	0.19	0.24	0.27	0.31	0.34	0.37
16 hrs.	0.17	0.21	0.24	0.28	0.31	0.34
18 hrs.	0.16	0.19	0.22	0.25	0.28	0.31
20 hrs.	0.14	0.18	0.20	0.23	0.26	0.28
24 hrs.	0.13	0.15	0.18	0.20	0.22	0.24

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.59 DRAINAGE SYSTEM OVERFLOW DESIGN

Drainage system shall have adequate capacity to safely and adequately convey the stormwater runoff from all upstream tributary areas through the development under consideration for a storm of 100-year design return period calculated on the basis of the upstream land in its present state of development. An allowance, equivalent to the reduction in flow rate provided, shall be made for upstream detention when the upstream detention and release rate previously been approved by the Commission or Board and evidence of its construction can be shown.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.60 DETERMINATION OF STORAGE VOLUME; METHODS

(A) For areas of ten (10) acres or less, the rational method may be used to determine the required volume of stormwater storage. The following eleven step procedure may be used to determine the required volume of storage. Other design methods may also be used, subject to approval of the Commission; and as described herein.

- (1) Determine total drainage area in acres "A;"
- (2) Determine composite runoff coefficient "C" based on-existing land use (undeveloped);
- (3) Determine time of concentration "tc" in minutes based on existing conditions;
- (4) Determine rainfall intensity "I" in inches per hour, based on time of concentration and using data given herein for the three-year return period;
- (5) Compute runoff based on existing land use (undeveloped), and three-year return period:

$$Q_u = CUIUA$$

- (6) Determine composite runoff coefficient "Cd" based on developed conditions and a 100-year return period;
- (7) Determine the 100-year return period rainfall intensity "Id" for various storm durations "td" up through the time of concentration for the developed area using information herein;
- (8) Determine developed inflow rates "Qd" for various storm durations "td" measured in hours;

$$Q_d = C_d I_d A$$

- (9) Compute a storage rate "Std" for various storm durations "td" up through the time of concentration of the developed area; and

$$S_{td} = Q_d - Q_u$$

- (10) Compute required storage volume "Sr" in acre-feet for each storm duration "td" hours;

$$S_r = S_{td} (td/12)$$

(11) Select the largest storage volume computed in subsection (10) for detention basin design.

(B) Methods other than the rational method for determining runoff and routing of stormwater may be used to determine the storage volume required to control stormwater runoff. The procedures or methods used must receive the prior approval of the Commission. The ILLUDAS, TR20 and TR-55 models are approved by the Commission for appropriate use in analysis of the runoff and routing of stormwater. The use of these models or other approved procedures can be defined in a seven-step procedure to determine the required storage volume of the detention basin.

- (1) Calibrate the hydrologic/hydraulic model that is to be used for prediction of runoff and routing of stormwater;
- (2) For each storm duration listed herein, perform steps three through six;
- (3) Determine the three-year, undeveloped peak flow. (Denote this flow by Q^3_u);
- (4) Determine the 100-year runoff hydrograph (H^{100}_d) for developed conditions;
- (5) Determine the hydrograph that must be stored (H^{100}_d) by subtracting a flow up to Q^3_u from the hydrograph (H^{100}_d) found in subsection (4);
- (6) Determine the volume of water (V_5) to be stored by calculating the area under the hydrograph (H^{100}_d); and
- (7) The detention basin must be designed to store the largest volume (V_5) found for any storm duration analyzed in subsection (6).
- (8) Retention/Detention systems will be designed to store the 10-year developed CN and have a release rate equal to 0.1 acres per acre of development.
- (9) Retention/Detention systems will be designed to store the 100-year developed CN and have a release rate equal to 0.3 acres per acre of development.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.61 BASIN REQUIREMENTS

(A) Basins shall be constructed to detain temporarily the stormwater runoff which exceeds the maximum peak flow rate authorized by this chapter. The volume of storage provided in these basins, together with such storage as may be authorized in other on-site facilities shall be sufficient to control excess runoff from the 100-year storm. The following design principles shall be observed:

- (1) The maximum volume of water stored and subsequently released at the design release rate shall not result in a storage duration in excess of 12 hours unless additional storms occur within the period.
- (2) The maximum planned depth of stormwater stored (without a permanent pool) shall not exceed five (5) feet.
- (3) All stormwater detention facilities shall be separated by not less than 50 feet from any building or structure to be occupied and if located within seventy-five (75) feet of a roadway or sidewalk must have a vehicular barrier in a design acceptable to the Commission. In addition, all detention facilities will have an emergency access ramp.
- (4) All excavated excess soil may be spread so as to provide for aesthetic and recreational features such as sliding hills, sports fields and the like. Slopes no steeper than eight horizontal to one vertical for safety, erosion control, stability and ease of maintenance shall be permitted.
- (5) Safety screens having a maximum opening of four inches shall be provided for any pipe or opening to prevent children or large animals from crawling into the structures.
- (6) All basins must be surrounded by a chain link fence surrounding its perimeter. The property owner of the basin may apply for a waiver of this requirement if the owner provides proof of liability insurance and executes an indemnity agreement in which the owner agrees to indemnify and hold Montgomery County, and its boards, officers, employees, and agents harmless from any liability for claims asserted by persons or other entities because of personal injury, death, property damage or other injury relating to the basin. Any such indemnity obligation will be permanent and will become an obligation of the owner of the land in perpetuity. In addition, any such indemnity obligation must appear as a covenant running with the land and be recorded in the Office of the Recorder of Montgomery County. The Commission, in its discretion, may grant or deny any such waiver request. As a condition of any approval of waiver, the Commission may require the owner to post a performance bond in order to secure its faithful performance. Danger signs shall be mounted at appropriate locations to warn of deep water, possible flooding conditions during storm periods and other dangers that exist.
- (7) Outlet control structures shall be constructed and locate 0.5 feet above an approved receiving ditch or storm sewer system and be designed to operate as simply as possible and shall require little or no maintenance and/or attention for proper operation. They shall limit discharges into existing or planned downstream channels or conduits so as not to exceed the predetermined maximum authorized peak flow rate.

(8) Emergency overflow facilities such as a weir or spillway shall be provided for the release of exceptional storm runoffs or in emergency conditions should the normal discharge devices become totally or partially inoperative. The overflow facility shall be of the design that its operation is automatic and does not require manual attention.

(9) Grass or other suitable vegetative cover shall be provided throughout the entire basin area. Grass should be cut regularly at approximately monthly intervals during the growing season or as required.

(10) Debris and trash removal and other necessary maintenance shall be performed on a regular basis to assure continued operation in conformance to design.

(11) A report shall be submitted to the Commission describing the proposed development; the current land use conditions; the method of hydraulic and hydrologic analysis used, including any assumptions or special conditions; the results of the analysis; and the recommended drainage control facilities. Hydraulic and hydrologic calculations, including input and output files, shall be included as appendices to the report.

(12) The minimum size for a retention/detention restrictor orifice is 6-inches.

(B) Detention basins which will not contain a permanent pool of water shall comply with the following requirements:

(1) Provisions shall be incorporated to facilitate complete interior drainage of detention basins, to include the provisions of natural grades to outlet structures, longitudinal and transverse grades to perimeter drainage facilities, paved gutters or the installation of subsurface drains.

(2) The detention basin shall, whenever possible, be designed to serve a secondary or multi-purpose function. Recreational facilities, aesthetic qualities (open spaces) or other types of use shall be considered in planning the detention facility.

(3) All detention basins will be designed and constructed with a flow line slope of 1.0%. Flow line slopes between 0.5% and 1.0% shall install a minimum of a four-inch perforated underdrain. The perforated underdrain shall be constructed in a chevron pattern spaced on 30-foot centers across the bottom of the detention basin and shall be located at least one (1) foot below the bottom of the basin. The perforated underdrain will be connected to the detention basin outlet structure. If the flow line slope is less than 0.5%, a concrete paved ditch will be installed, and the paved ditch will be constructed in a chevron pattern spaced on 30-foot centers across the bottom of the basin.

(C) Since a retention basin by definition will contain a permanent pool of water, all the items required for detention storage shall apply except that the system of drains with a positive gravity outlet required to maintain a detention basin will not be required. A controlled positive outlet will be required to maintain the design water level in the retention basin and provide required detention storage above the design water level. However, the following additional conditions shall apply:

(1) Basins designed with permanent pools or containing permanent ponds shall have a water area of at least one-half acre, a minimum depth of eight (8) feet, and a safety ledge which meets the requirements of subsection (3). If fish are to be maintained in the pond, a minimum depth of approximately ten feet shall be maintained over at least 25% of the pond area. The remaining pond area shall have no extensive shallow areas, except as required by division (C)(3) below.

(2) In excavated ponds, the underwater side slopes in the pond shall be stable. IN the case of valley storage, natural slopes may be considered to be stable.

(3) A safety ledge four to six feet in width is required and must be installed in all ponds approximately 30 to 36 inches below the permanent water level. In addition, a similar maintenance ledge 12 to one inches above the permanent water line shall be provided. The slope between the two ledges shall be stable and of a material such as stone or riprap which will prevent erosion due to wave action.

(4) A safety ramp exit from the pond is required in all cases and shall have a minimum width of 20 feet and exit slope to six horizontal to one vertical. The ramp shall be of a material that will prevent its deterioration due to vehicle use and/or wave action.

(5) Periodic maintenance is required in ponds to control weed and larval growth. The pond shall also be designed to provide for the easy removal of sediment which will accumulate during periods of pond operation. A means of maintaining the designed water level of the pond during prolonged periods of dry weather is also required.

(6) For emergency use, basin cleaning or shoreline maintenance, facilities shall be provided or plans prepared for auxiliary equipment to permit emptying and drainage.

(7) Facilities to enhance and maintain pond water quality shall be provided, if required to meet applicable water quality standards. Design calculations to substantiate the effectiveness of these aeration facilities shall be submitted with final engineering plans. Agreements for the perpetual operation and maintenance of aeration facilities shall be submitted with final engineering plans. Agreements for the perpetual operation and maintenance of aeration facilities shall be prepared to the satisfaction of the Board. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.62 PARKING LOT STORAGE

Paved parking lots may be designed to provide temporary detention storage of stormwater on all or a portion of their surfaces. Outlets will be designed so as to empty the stored waters slowly. Depth of storage must be limited to a maximum depth of six inches so as to prevent damage to parked vehicles and so that access to parked vehicles is not impaired. Ponding should, in general, be confined to those positions of the parking lots furthest from the area served. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.63 FACILITY FINANCIAL RESPONSIBILITY

The construction cost of stormwater control systems and facilities as required by this chapter shall be accepted as part of the cost of land development. If general public use of the facility can be demonstrated, negotiations for public participation in the cost of such development may be considered.
(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.64 FACILITY MAINTENANCE RESPONSIBILITY

(A) Facility maintenance responsibility of detention/retention facilities during construction and thereafter, shall be the responsibility of the land developer/owner. Assignment of responsibility for maintaining facilities serving more than one lot or holding shall be documented by appropriate covenants to property deeds, unless responsibility is formally accepted by a public body, and shall be determined before the final drainage plans are approved.

(B) Stormwater detention and retention basins may be donated to the county or other unit government designated by the county, for ownership and permanent maintenance providing:

- (1) The county or other governmental unit is willing to accept responsibility;
- (2) The facility has been designed and constructed according to all applicable provisions of this chapter;
- (3) All improvements have been constructed, approved and accepted by the county for the land area served by the drainage basin;
- (4) Retention ponds containing a permanent pool of water have all slopes between the riprap and high water line sodded and the remaining land area hydro seeded; are equipped with electrically driven aeration devices, if required to maintain proper aerobic conditions and sustain aquatic life; have a four-foot-wide crushed limestone walkway at the high water line entirely around the body of water; provide suitable public access

acceptable to the responsible governmental agency; and have the high water line not closer than 75 feet to any property line; and

(5) Dry detention ponds shall have all slopes, bottom of the basin and area above the high water line hydro seeded and shall have the high-water line not closer than 50 feet to any development boundary. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.65 INSPECTIONS

All public and privately owned detention and retention storage facilities may be inspected by representatives of the county not less than once every two years. A certified inspection report covering physical conditions, available storage capacity and operational condition of key facility elements will be provided to the owner.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.66 CORRECTIVE MEASURES

If deficiencies are found by the Inspector, the persons responsible for the detention/retention facility will be required to take the necessary measures to correct the deficiencies. If the persons responsible fail to do so, the county will undertake the work and collect from the persons responsible using lien rights, if necessary.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.67 JOINT DEVELOPMENT

Stormwater control systems may be planned and constructed jointly by two or more developers as long as compliance with this chapter is maintained. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.68 INSTALLATIONS

(A) Runoff and erosion control system shall be installed as soon as possible during the course of site development. Detention/retention basins shall be designed with an additional 6% of available capacity to allow for sediment accumulation resulting from development and to permit the pond to function for reasonable periods between cleanings.

(1) Platted residential developments must include a perforated underdrain for each lot which connects to the sump pump(s). This underdrain system must be designed by an engineer and sized to handle anticipated flows. No sanitary sewage or garage floor drains or roof drains may be connected to the underdrain system. This underdrain system must connect to the subdivision storm water system. Perimeter septic system tile may connect to the perforated underdrain system with the approval of the Montgomery

County Health Department. A professional engineer will design the perforated underdrain system to ensure that water will not back flow into the septic system.

(2) Platted residential developments must submit a report by a licensed geotechnical engineer which includes a determination of the anticipated ground water table. The engineer will recommend appropriate spacing for the soil borings, and soil boring locations, soil profile and ground water elevations will be shown on the recorded plat.

(3) Platted residential developments must file an application with the Drainage Board and cooperate with the Drainage Board in designating the storm water system for the subdivision, including retention basins, detention basins, storm sewers, storm inlets, manholes and underdrains, to be a separate legal drain which is subject to the legal jurisdiction of the Drainage Board. The Board will determine the appropriate regulated drain rate for each development. No approvals may be granted for development and no permits may be issued until this designation has been approved by the Board.

(4) The owner will provide the Board with As-Built drawings of all storm water systems, including but not limited to all basins. These As-Built drawings must also be submitted to the Montgomery County Mapping Department in an electronic format consistent with the Mapping Department's layering system. All storm water structures must be constructed to within 0.083 feet of the approved construction documents. All basins must be constructed to within 0.167 feet of the approved construction documents. Drainage systems may not be constructed below the drainage outlet for the development.

(5) Platted residential developments will provide a 100-year flood route. Residence finish floor and/or crawl space vents will be set a minimum of 1 foot above the 100-year flood route elevation.

(6) Plated residential developments will provide a 20-foot wide drainage easement along the perimeter of the subdivision and along all lot rear yards abutting another residential lot.

(7) Un-platted residential lots may not have impervious area exceeding 25%.

(B) Basins should be designed to collect sediment and debris in specific locations so that removal costs are kept to a minimum.

(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.69 DETENTION FACILITIES IN FLOODPLAINS

If detention storage is provided within a floodplain, only the net increase in storage volume above that which naturally existed on the floodplain shall be credited to the development. No credit will be granted for volumes below the elevation of the regulatory

flood at the location unless compensatory storage is also provided. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.70 OFF-SITE DRAINAGE

All retention and detention and storm sewer systems will accommodate off-site drainage areas. Off-site drainage may be diverted through and/or around the proposed development in a separate drainage system. When the allowable runoff is released in an area that is susceptible to flooding, the developer may be required to construct appropriate storm drains through the area to avert increased flood hazard caused by the concentration of allowable runoff at one point instead of the natural overland distribution. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

PLANS AND DETERMINATIONS

§154.80 CERTIFICATIONS REQUIRED

(A) After completion of the project and before final approval and acceptance can be made, a professionally prepared and certified "record set" of plans shall be submitted to the County Engineer and Commission for review.

(B) These plans shall include all pertinent data relevant to the completed storm drainage system and shall include:

- (1) Pipe size and pipe material;
- (2) Invert elevations;
- (3) Top rim elevations;
- (4) Lengths of all pipe structures;
- (5) Data and calculations showing detention basin storage volume; and
- (6) Certified statement on plans stating the completed storm drainage system substantially complies with construction plans as approved by the Board.

(C) All submitted plans shall be reviewed for compliance within 30 days after submission to the County Engineer and Commission. If notice of noncompliance is not given to the applicant by the County Engineer within 30 days of submission of the plans, the plans shall be deemed approved and accepted. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.81 CHANGES IN PLAN

Any revision, significant change or deviation in the detailed plans and specifications after formal approval by the Commission, and if a subdivision, also by the Board, shall be filed in duplicate with and approved by the Commission and/or Board prior to implementation of the revision or change. Copies of the revisions or changes, if approved, shall be attached to the original plans and specifications. (Ord. 90-5, passed 8-30-9-; Am. Ord. passed 2-16-99)

§154.82 DETERMINATION OF IMPACT DRAINAGE AREAS

(A) The Board is authorized, but is not required to classify certain geographical areas as impact drainage areas and to enact and promulgate regulations which are generally applied. In determining impact drainage areas, the Board shall consider the factors as topography, soil type, capacity of existing regulated drains and distance from adequate drainage facility.

(B) The following areas shall be designated as impact drainage areas, unless good reason for not including them is presented to the Drainage Board:

- (1) A floodway or floodplain, as designated by the State Department of Natural Resources;
- (2) Land within 75 feet of each bank of any regulated drain;
- (3) Land within 75 feet of the centerline of any regulated drain tile; and
- (4) Land where there is not an adequate outlet, taking into consideration the capacity and depth of the outlet, may be designated as an impact drainage area by resolution of the Board. Special requirements for development within any impact drainage area shall be included in the resolution (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.83 SUMP PUMPS

(A) Sump pumps installed to receive and discharge groundwaters or other stormwaters shall be connected to the designated underdrain system as provided for in Section 154.68(A)(1).

(B) Sump pumps installed to receive and discharge floor drain flow or other sanitary sewage shall be connected to the sanitary sewers. A sump pump shall be used for one function only, either the discharge of stormwater or the discharge of sanitary sewage.
(Ord. 90-5, passed 8-30-90; Am. Ord. 2-16-99)

§154.84 DOWNSPOUTS

All downspouts or roof drains shall discharge onto the ground or be connected to the storm sewer. No downspouts or roof drains shall be connected to the sanitary sewers or sump pump underdrain system. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.85 FOOTING DRAINS

Footing drains shall be connected to storm sewers where possible or designated storm drainage channels. No footing drains or drainage tile shall be connected to the sanitary sewer. (Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)

§154.86 BASEMENT FLOOR DRAINS

Basement floor drains shall be connected to the sanitary sewers.
(Ord. 90-5, passed 8-30-90; Am. Ord. passed 2-16-99)